

COMPRESSION MEMBERS

Compression Members: are Structural elements that are subjected to axial compressive force only, that is, the loads are applied along a longitudinal axis through the centroid of the member

The stress in the compression member cross-section can be calculated as

$$\sigma = f_{cr} = P/A$$

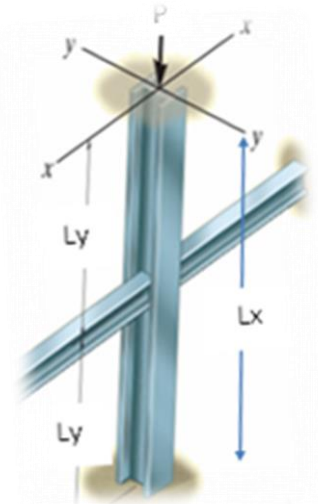
$\sigma = f_{cr}$ = normal compressive stress

P= axial compressive force.

A= gross sectional area

Column is a vertical member whose primary function is to support vertical loads.

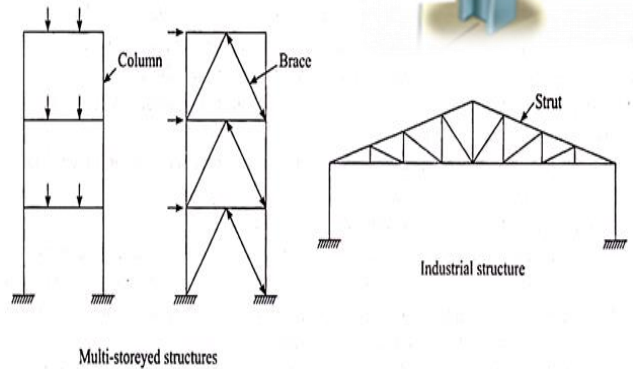
In many cases compression members are also resisting bending, and in these cases the member is a beam column.



Smaller compression members not classified as column are sometimes referred as Struts or posts.

Thus Compression members are found as:-

- 1- Columns in buildings .
- 2- Piers in bridges .
- 3- Trusses (Top chords) .
- 4- Bracing members .

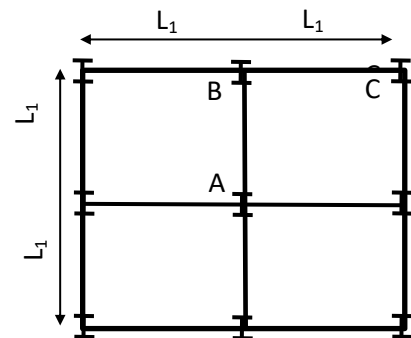


The most common type of compression member in building and bridges is the column.

Column can be classified according to:

1- Type of Loading

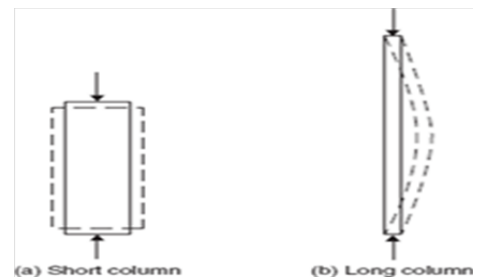
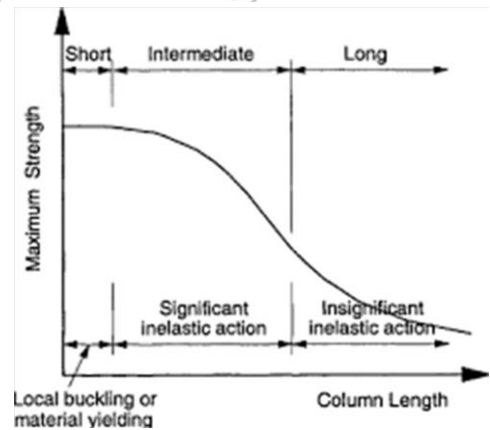
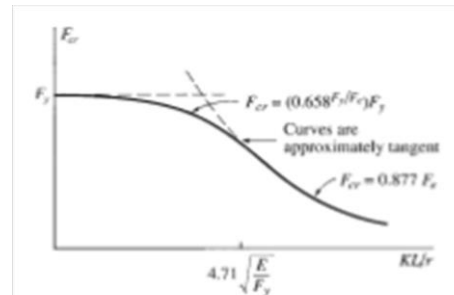
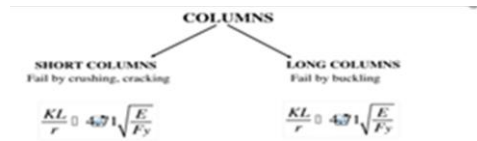
- Axially Loaded (Column), Like col. (A).
- Eccentrically Loaded like (Beam Column)
- Beam Col. with Uniaxial moment, Like col.(B)
- Beam Col. with Biaxial moment, Like col.(C)



2- Slenderness Ratio for column (KI/r)

- Long Column
- Short Column
- Intermediate Column

Short Column	Long Column
A column is considered to be short if the ratio of effective length to its least lateral dimension (kl/r) is less than or equal to $4.71\sqrt{E/fy}$.	A column is considered to be long if the ratio of effective length of column to its least lateral dimension (kl/r) is greater than $4.71\sqrt{E/fy}$.
Buckling tendency is very low.	Long and slender columns buckle easily.
The load carrying capacity is high as compared to long column of the same cross sectional area.	The load carrying capacity is less as compared to short column of the same cross sectional area.
The failure of the short column is by yielding.	The column generally fails in buckling.



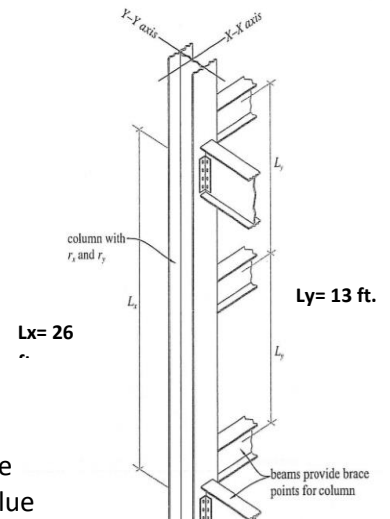
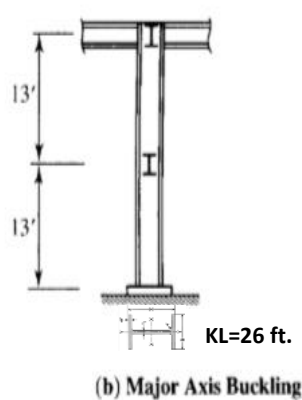
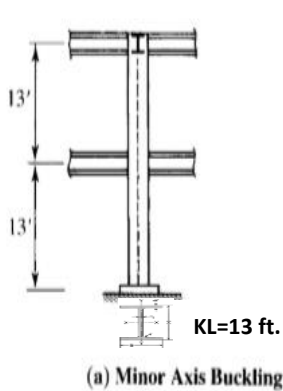
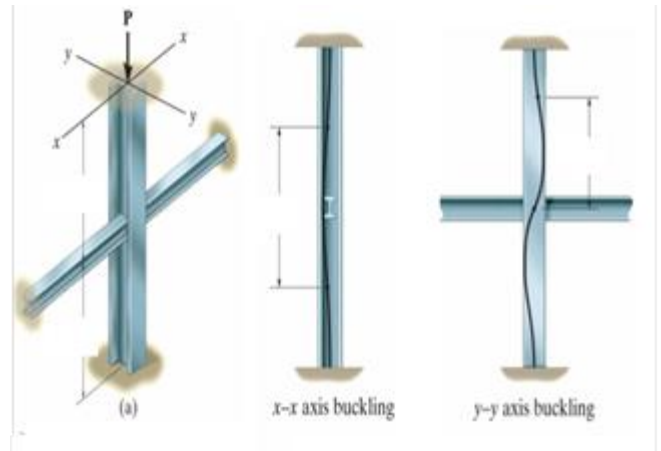
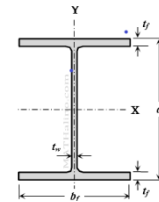
EFFECTIVE LENGTH OF COLUMNS IN FRAMES

The effective length (KL) of a column is defined as the distance between successive inflection points or zero point moment.

Where

K: depending on the end condition of the ends and sway condition (brace or not brace)

L: refers to the length between the brace points, it's divided in two parts according to X axis and Y axis and the number of brace points in each direction

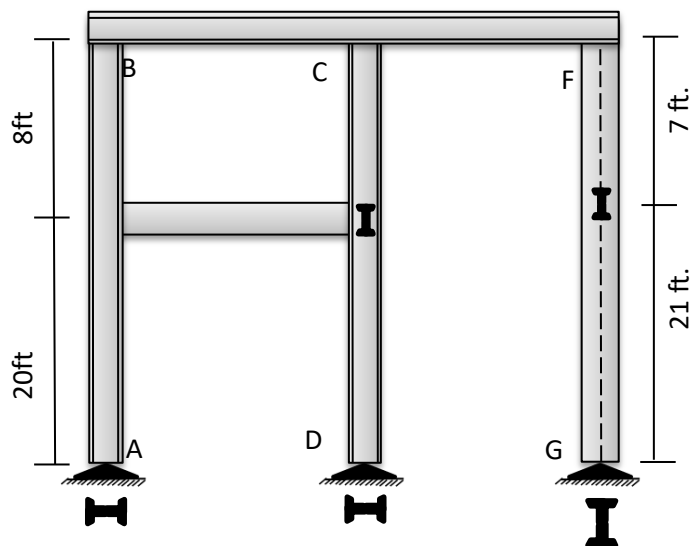


Note

If the web was bracing then (L_y) will be divided, then take the larger value
 If the flange was bracing then (L_x) will be divided, then take the larger value
 If both web and flange were bracing then (L_y & L_x) will be divided, then take the larger value

Homework

Find the length of each column for the frame shown below?



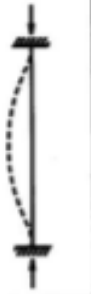





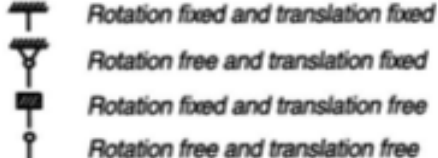
Where K is usually determined by one of two methods:

1. AISC, Table C-C2.2: This table is used for isolated columns that are not part of a continuous frame. In Table C-C2.2, P.P 240 part Two a through c represents columns in braced frames; while d through f represent columns in unbraced frames.

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CALCULATION OF REQUIRED STRENGTHS

[Comm. C2.

TABLE C-C2.2 Approximate Values of Effective Length Factor, K						
Buckled shape of column is shown by dashed line.	(a) 	(b) 	(c) 	(d) 	(e) 	(f) 
Theoretical K value	0.5	0.7	1.0	1.0	2.0	2.0
Recommended design value when ideal conditions are approximated	0.65	0.80	1.2	1.0	2.10	2.0
End condition code						

Example

Find the effective length factor of (W 12 x 50) columns has length is equal to 20 ft. Assume the column is pin- pin the weak direction (For minor axis) and the two ends connections for strong direction (Major axis) are:

- 1-Pin-pin contention in both directions
- 2-Fixed in -fixed contention
- 3 - Fixed in -pin contention
- 4-Fixed-free to translate and rotated (rotation free and translation free)

Solution

Since the Y_Y axis (weak axis) is pin-pin in both direction

$K_y = 1$

$K_x =$ according to the conditions of the ends

- 1- (pin - pin end connection)

$K_x = 0.1$ (pin - pin end connection)

$K_y = 1.0$ (pin - pin end connection)

$L_x = l_y$ (the unsupported length for two axis is 20 ft)

$Kl_x = Kl_y = 1 \times 20 = 20$ ft

- 2- (fixed - fixed end connection)

$K_x = 0.65$ (pin - fixed end connection)

$K_y = 1.0$ (pin - pin end connection)

$L_x = l_y$ (the unsupported length for two axis is 20 ft)

$Kl_x = 0.65 \times 20 = 13$ ft

$Kl_y = 1 \times 20 = 20$ ft)

- 3- (fixed - pin end connection)

$K_x = 0.8$ (pin - fixed end connection)

$K_y = 1.0$ (pin - pin end connection)

$L_x = l_y$ (the unsupported length for two axis is 20 ft)

$Kl_x = 0.8 \times 20 = 16$ ft

$Kl_y = 1 \times 20 = 20$ ft

- 4 (-Fixed-free to translate only (rotation fixed and translation free)

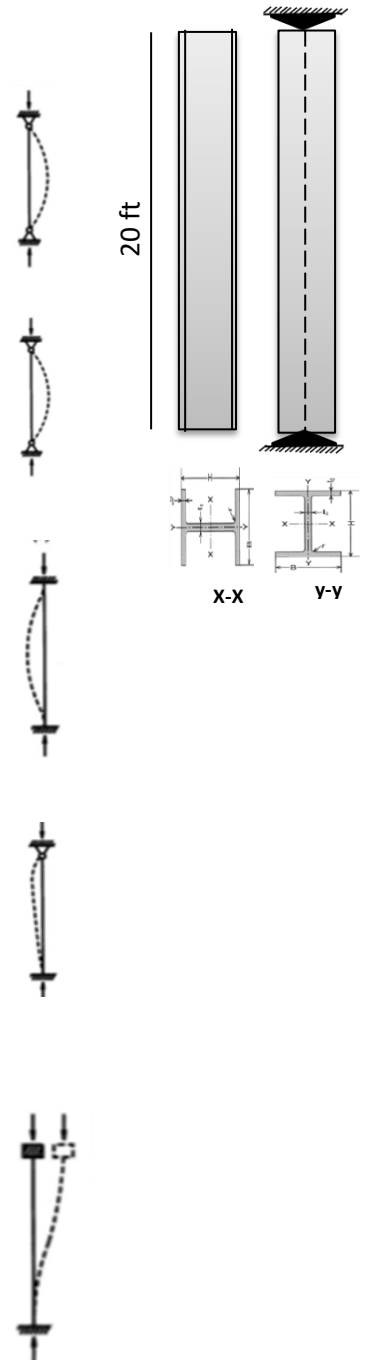
$K_x = 1.2$ (pin - fixed end connection) in the X-X axis

$K_y = 1.0$ (pin - pin end connection) in the y-y axis

$L_x = l_y$ (the unsupported length for two axis is 20 ft)

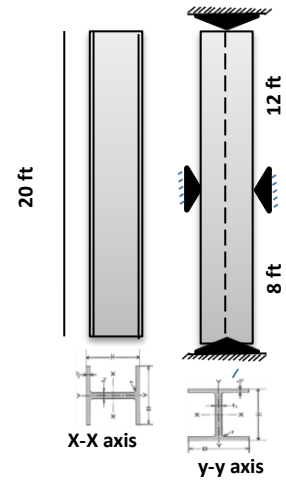
$Kl_x = 1.2 \times 20 = 24$ ft

$Kl_y = 1 \times 20 = 20$ ft



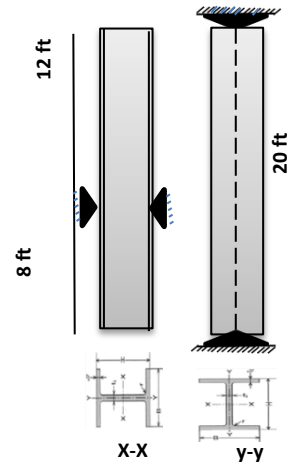
Homework 1

Re solve the above example if
The col. has supported at 12 ft in the weak direction



Homework 2

Re solve the above example if
The col. has supported at 12 ft in the strong direction



Homework 3

Re solve the above example if
The col. has supported in both directions

