

FIGURE P3.2-3

- 3.2-4** A PL $\frac{3}{8} \times 6$ tension member is welded to a gusset plate as shown in Figure P3.2-4. The steel is A36. Assume that $A_e = A_g$ and compute the following.
- The design strength for LRFD.
 - The allowable strength for ASD.

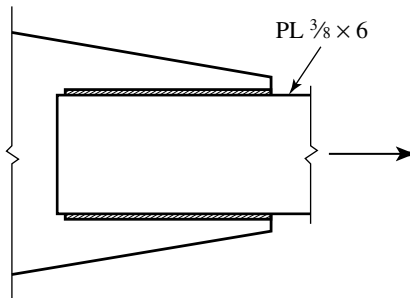


FIGURE P3.2-4

- 3.2-5** The tension member shown in Figure P3.2-5 is a PL $\frac{1}{2} \times 8$ of A36 steel. The member is connected to a gusset plate with $1\frac{1}{8}$ inch-diameter bolts. It is subjected to the dead and live loads shown. Does this member have enough strength? Assume that $A_e = A_n$.
- Use LRFD.
 - Use ASD.

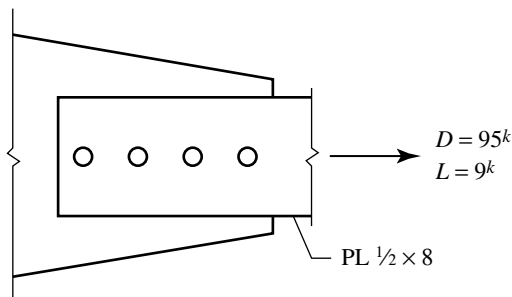


FIGURE P3.2-5

- 3.2-6** A double-angle tension member, $2L\ 3 \times 2 \times \frac{1}{4}$ LLBB, of A36 steel is subjected to a dead load of 12 kips and a live load of 36 kips. It is connected to a gusset plate with $\frac{3}{4}$ -inch-diameter bolts through the long legs. Does this member have enough strength? Assume that $A_e = 0.85A_n$.
- Use LRFD.
 - Use ASD.

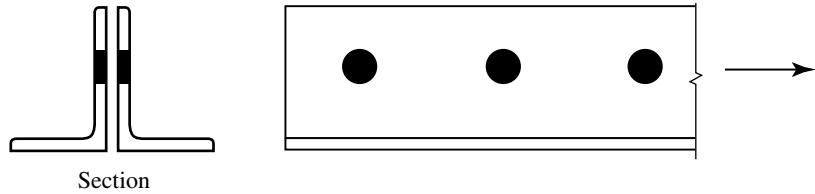


FIGURE P3.2-6

- 3.2-7** A $C8 \times 11.5$ is connected to a gusset plate with $\frac{7}{8}$ -inch-diameter bolts as shown in Figure P3.2-7. The steel is A572 Grade 50. If the member is subjected to dead load and live load only, what is the total service load capacity if the live-to-dead load ratio is 3? Assume that $A_e = 0.85A_n$.
- Use LRFD.
 - Use ASD.

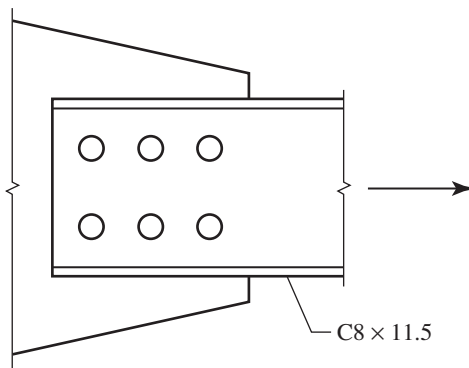


FIGURE P3.2-7

Effective area

- 3.3-1** Determine the effective area A_e for each case shown in Figure P3.3-1.