

## EXAMPLE 4.8

Select a W18 shape of A992 steel that can resist a service dead load of 100 kips and a service live load of 300 kips. The effective length  $KL$  is 26 feet.

### LRFD SOLUTION

$P_u = 1.2D + 1.6L = 1.2(100) + 1.6(300) = 600$  kips  
 Try  $F_{cr} = 33$  ksi (an arbitrary choice of two-thirds  $F_y$ ):

$$\text{Required } A_g = \frac{P_u}{\phi_c F_{cr}} = \frac{600}{0.90(33)} = 20.2 \text{ in.}^2$$

Try a W18  $\times$  71:

$$A_g = 20.9 \text{ in.}^2 > 20.2 \text{ in.}^2 \quad (\text{OK})$$

$$\frac{KL}{r_{\min}} = \frac{26 \times 12}{1.70} = 183.5 < 200 \quad (\text{OK})$$

$$F_e = \frac{\pi^2 E}{(KL/r)^2} = \frac{\pi^2 (29,000)}{(183.5)^2} = 8.5 \text{ ksi}$$

$$4.71 \sqrt{\frac{E}{F_y}} = 4.71 \sqrt{\frac{29,000}{50}} = 113$$

Since  $\frac{KL}{r} > 4.71 \sqrt{\frac{E}{F_y}}$ , AISC Equation E3-3 applies.

$$F_{cr} = 0.877 F_e = 0.877(8.5) = 7.455 \text{ ksi}$$

$$\phi_c P_n = \phi_c F_{cr} A_g = 0.90(7.455)(20.9) = 140 \text{ kips} < 600 \text{ kips} \quad (\text{N.G.})$$

Because the initial estimate of  $F_{cr}$  was so far off, assume a value about halfway between 33 and 7.455 ksi. Try  $F_{cr} = 20$  ksi.

$$\text{Required } A_g = \frac{P_u}{\phi_c F_{cr}} = \frac{600}{0.90(20)} = 33.3 \text{ in.}^2$$

Try a W18  $\times$  119:

$$A_g = 35.1 \text{ in.}^2 > 33.3 \text{ in.}^2 \quad (\text{OK})$$

$$\frac{KL}{r_{\min}} = \frac{26 \times 12}{2.69} = 116.0 < 200 \quad (\text{OK})$$

$$F_e = \frac{\pi^2 E}{(KL/r)^2} = \frac{\pi^2 (29,000)}{(116.0)^2} = 21.27 \text{ ksi}$$

Since  $\frac{KL}{r} > 4.71\sqrt{\frac{E}{F_y}} = 113$ , AISC Equation E3-3 applies.

$$F_{cr} = 0.877F_e = 0.877(21.27) = 18.65 \text{ ksi}$$

$$\phi_c P_n = \phi_c F_{cr} A_g = 0.90(18.65)(35.1) = 589 \text{ kips} < 600 \text{ kips} \quad (\text{N.G.})$$

This is very close, so try the next larger size.

Try a W18 × 130:

$$A_g = 38.3 \text{ in.}^2$$

$$\frac{KL}{r_{\min}} = \frac{26 \times 12}{2.70} = 115.6 < 200 \quad (\text{OK})$$

$$F_e = \frac{\pi^2 E}{(KL/r)^2} = \frac{\pi^2 (29,000)}{(115.6)^2} = 21.42 \text{ ksi}$$

Since  $\frac{KL}{r} > 4.71\sqrt{\frac{E}{F_y}} = 113$ , AISC Equation E3-3 applies.

$$F_{cr} = 0.877F_e = 0.877(21.42) = 18.79 \text{ ksi}$$

$$\phi_c P_n = \phi_c F_{cr} A_g = 0.90(18.79)(38.3) = 648 \text{ kips} > 600 \text{ kips} \quad (\text{OK.})$$

This shape is not slender (there is no footnote in the dimensions and properties table to indicate that it is), so local buckling does not have to be investigated.

**ANSWER** Use a W18 × 130.

### ASD SOLUTION

The ASD solution procedure is essentially the same as for LRFD, and the same trial values of  $F_{cr}$  will be used here.

$$P_a = D + L = 100 + 300 = 400 \text{ kips}$$

Try  $F_{cr} = 33 \text{ ksi}$  (an arbitrary choice of two-thirds  $F_y$ ):

$$\text{Required } A_g = \frac{P_a}{0.6F_{cr}} = \frac{400}{0.6(33)} = 20.2 \text{ in.}^2$$

Try a W18 × 71:

$$A_g = 20.9 \text{ in.}^2 > 20.2 \text{ in.}^2 \quad (\text{OK})$$

$$\frac{KL}{r_{\min}} = \frac{26 \times 12}{1.70} = 183.5 < 200 \quad (\text{OK})$$