

of 5.413 in.^2 should be multiplied by $^{10}/_9$ to obtain a net area that can be compared with those lines that resist the full load. Use $A_n = 5.413(^{10}/_9) = 6.014 \text{ in.}^2$ For line *abcdeg*,

$$g_{cd} = 3 + 2.25 - 0.5 = 4.75 \text{ in.}$$

$$\begin{aligned} A_n &= 6.80 - 0.5(1.0) - 0.5 \left[1.0 - \frac{(1.5)^2}{4(2.5)} \right] - 0.5 \left[1.0 - \frac{(1.5)^2}{4(4.75)} \right] - 0.5 \left[1.0 - \frac{(1.5)^2}{4(3)} \right] \\ &= 5.065 \text{ in.}^2 \end{aligned}$$

The last case controls; use

$$A_n = 5.065 \text{ in.}^2$$

Both legs of the angle are connected, so

$$A_e = A_n = 5.065 \text{ in.}^2$$

The nominal strength based on fracture is

$$P_n = F_u A_e = 58(5.065) = 293.8 \text{ kips}$$

The nominal strength based on yielding is

$$P_n = F_y A_g = 36(6.80) = 244.8 \text{ kips}$$

a. The design strength based on fracture is

$$\phi_t P_n = 0.75(293.8) = 220 \text{ kips}$$

The design strength based on yielding is

$$\phi_t P_n = 0.90(244.8) = 220 \text{ kips}$$

ANSWER Design strength = 220 kips.

b. For the limit state of fracture, the allowable stress is

$$F_t = 0.5F_u = 0.5(58) = 29.0 \text{ ksi}$$

and the allowable strength is

$$F_t A_e = 29.0(5.065) = 147 \text{ kips}$$

For yielding,

$$F_t = 0.6F_y = 0.6(36) = 21.6 \text{ ksi}$$

$$F_t A_g = 21.6(6.80) = 147 \text{ kips}$$

ANSWER Allowable strength = 147 kips.

EXAMPLE 3.8

Determine the smallest net area for the American Standard Channel shown in Figure 3.18. The holes are for $5/8$ -inch-diameter bolts.

SOLUTION

$$A_n = A_g - \sum t_w \times (d \text{ or } d')$$

$$d = \text{bolt diameter} + \frac{1}{8} = \frac{5}{8} + \frac{1}{8} = \frac{3}{4} \text{ in.}$$

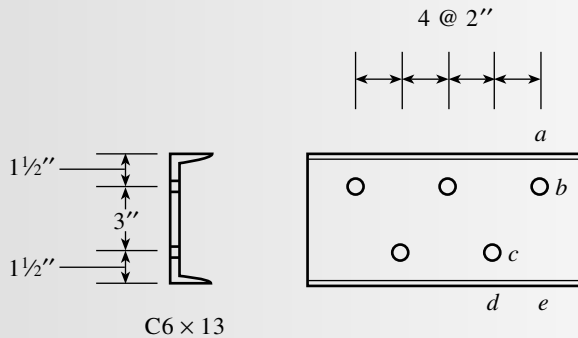
Line *abe*:

$$A_n = A_g - t_w d = 3.82 - 0.437 \left(\frac{3}{4} \right) = 3.49 \text{ in.}^2$$

Line *abcd*:

$$\begin{aligned} A_n &= A_g - t_w (d \text{ for hole at } b) - t_w (d' \text{ for hole at } c) \\ &= 3.82 - 0.437 \left(\frac{3}{4} \right) - 0.437 \left[\frac{3}{4} - \frac{(2)^2}{4(3)} \right] = 3.31 \text{ in.}^2 \end{aligned}$$

ANSWER Smallest net area = 3.31 in.²

FIGURE 3.18

When staggered holes are present in shapes other than angles, and the holes are in different elements of the cross section, the shape can still be visualized as a plate, even if it is an I-shape. The AISC Specification furnishes no guidance for gage lines crossing a “fold” when the different elements have different thicknesses. A method for handling this case is illustrated in Figure 3.19. In Example 3.8, all of the holes are in one element of the cross section, so this difficulty does not arise. Example 3.9 illustrates the case of staggered holes in different elements of an S-shape.