

Cap design

$$f'_c = 28 \text{ Mpa}, f_y = 428 \text{ Mpa}$$

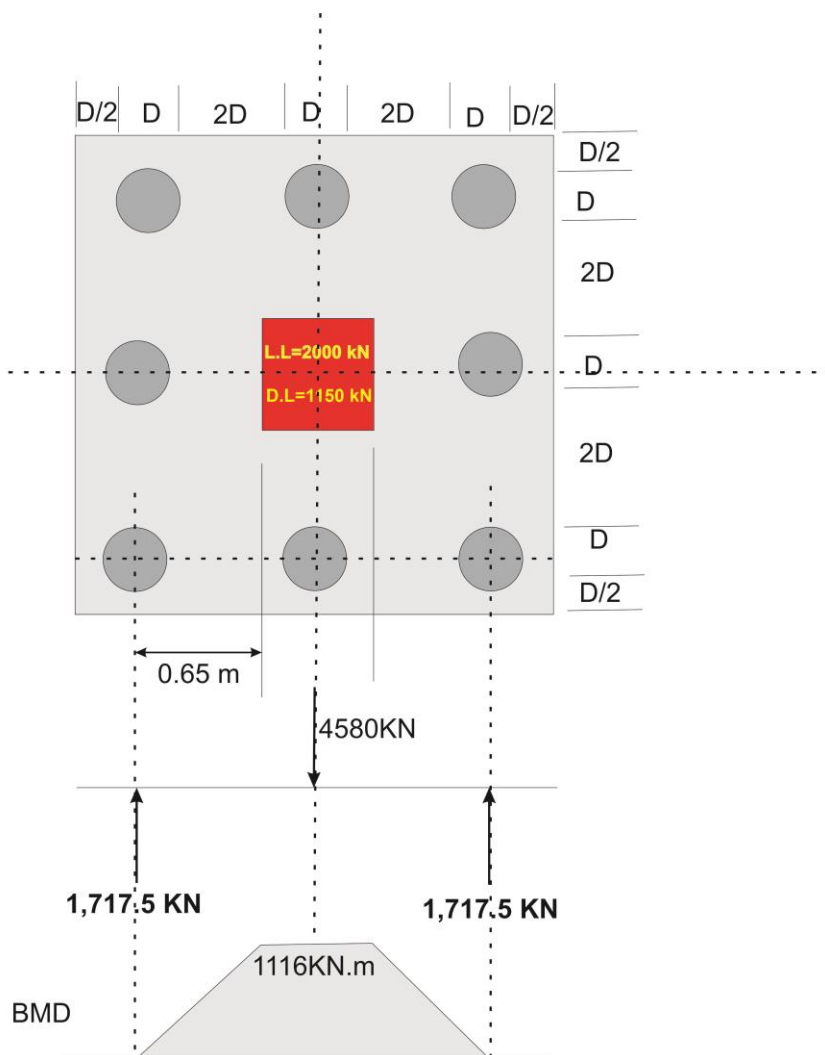
$$L.L = 2000 \text{ KN}, D.L = 1150 \text{ KN}$$

$D = 300\text{mm}$, Piles are embedded into the cap 100mm

Column dimensions 500mm*500mm

Cap dimension design

The plan dimensions of a pile cap depend on the number of piles that are needed to support the load. Number and arrangement of piles were determined from unfactored forces and moments transmitted to piles and permissible pile capacity selected through geotechnical engineering principles.



$$L = \frac{D}{2} + D + 2D + D + 2D + D + \frac{D}{2} = 8D = 8 * 300m = 2400mm$$

Pile Thickness

1- Two-way shear requirements around the column.

$$P_U = 1.2 D.L + 1.6 L.L$$

$$P_U = 1.2 * 1150 + 1.6 * 2000 = 4580 KN$$

$$U = b_o d \phi (0.34) \sqrt{f'_c}$$

$$4580 = 4(d + 0.5)d * 0.75 * 1000 * \sqrt{28}$$

$$0.2885 = (d + 0.5)d, \quad d = 0.34 \text{ m say } 350\text{mm} < 300 \text{ mm minimum cap thickness}$$

2- One-way and two-way shear requirements around the piles.

$$P_U \text{ for pile} = \frac{4580 KN}{8} = 572.5 KN$$

$$b_o = \pi(D + d)$$

$$b_o = \pi(0.3 + d)$$

$$P_U = b_o d \phi (0.34) \sqrt{f'_c}$$

$$572.5 = \pi(0.3 + d)d * 0.75 * 0.34 * 1000 * \sqrt{28}$$

$$(0.3 + d)d = 0.135, \quad d = 0.25\text{m} = 250 \text{ mm take } 350 \text{ mm}$$

Reinforcement calculation

The factored moment at the critical section is obtained by multiplying the reactions from three piles by the distance from the center of the piles to the critical section (at the face of the column) as follows:

$$M_U = 1717.5 KN * 0.65m = 1,116 KN.m$$

$$d = 0.35 \text{ m}$$

$$a = \frac{A_s f_y}{0.85 f_c b} = \frac{\rho d f_y}{0.85 f_c} \dots\dots\dots(1)$$

$$\rho = \frac{A_s}{bd} \dots\dots\dots(2)$$

$$M_U = \phi A_s f_y \left(d - \frac{a}{2} \right) \dots\dots\dots(3)$$

$$M_U = \phi \rho b d f_y \left(d - \frac{\rho d f_y}{1.7 f_c} \right) \dots\dots\dots(4)$$

$$a = \frac{A_s f_y}{0.85 f_c b} = \frac{\rho d f_y}{0.85 f_c} = \frac{\rho * 0.35 * 428}{0.85 * 28} = 6.294 \rho \text{ sub in (4)}$$

$$1116 = 0.85 * \rho * 0.35 * 2.4 * 428 * 100 \left(0.35 - \frac{\rho * 0.35 * 428}{1.7 * 28} \right)$$

$$0.00365 = \rho (0.35 - 3.147 \rho) , \rho = 0.01$$

From (2)

$0.01 = \frac{A_s}{2400 * 350}$, $A_s = 8400 \text{ mm}^2$, Bars No. = 17.14 say 18Ø25mm in two directions

