

Moment and shear coefficient ,ACI code method 8-3

For non-pre-stressed concrete

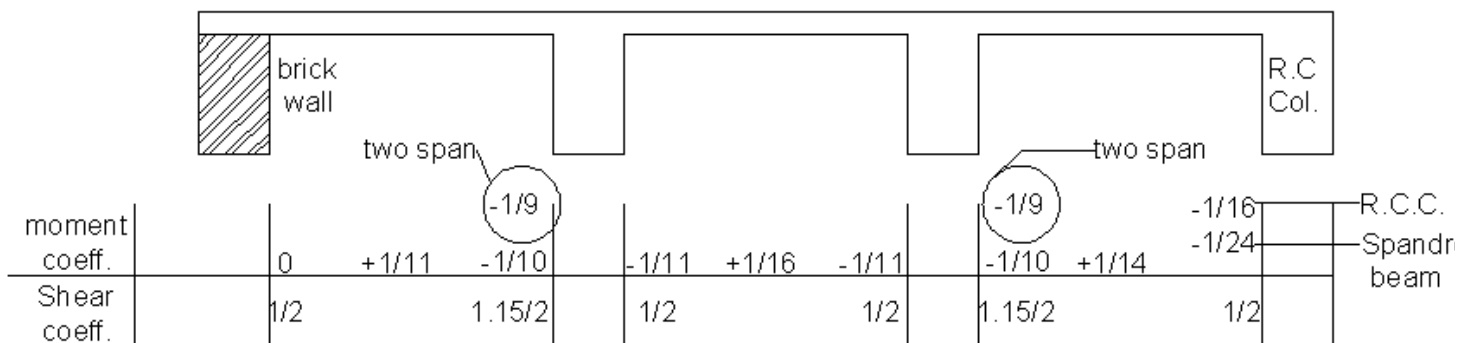
For continuous beam and one way slabs

Conditions

1. There are two or more span.
2. Spans are approximately equal with larger of two adjacent spans not greater than the shorter by more than (20%)
3. Loads are uniformly distributed
4. Unit live load not exceeding three times unit dead load
5. Members are prismatic

$$M = W_u \text{ coeff,}$$

$$W_u \text{ coeff}$$



8.3.3 — As an alternate to frame analysis, the following approximate moments and shears shall be permitted for design of continuous beams and one-way slabs (slabs reinforced to resist flexural stresses in only one direction), provided:

- (a) There are two or more spans;
- (b) Spans are approximately equal, with the larger of two adjacent spans not greater than the shorter by more than 20 percent;
- (c) Loads are uniformly distributed;
- (d) Unfactored live load, L , does not exceed three times unfactored dead load, D ; and
- (e) Members are prismatic.

For calculating negative moments, ℓ_n is taken as the average of the adjacent clear span lengths.

Positive moment

End spans

Discontinuous end
unrestrained..... $W_u \ell_n^2 / 11$

Discontinuous end integral
with support $W_u \ell_n^2 / 14$

Interior spans $W_u \ell_n^2 / 16$

Negative moments at exterior face
of first interior support

Two spans $W_u \ell_n^2 / 9$

More than two spans..... $W_u \ell_n^2 / 10$

Negative moment at other faces
of interior supports..... $W_u \ell_n^2 / 11$

Negative moment at face of all
supports for

Slabs with spans not exceeding
3 m; and beams where ratio of
sum of column stiffnesses to
beam stiffness exceeds eight at
each end of the span..... $W_u \ell_n^2 / 12$

Negative moment at interior face
of exterior support for members
built integrally with supports

Where support is spandrel beam $W_u \ell_n^2 / 24$

Where support is a column $W_u \ell_n^2 / 16$

Shear in end members at face of
first interior support..... $1.15 W_u \ell_n / 2$

Shear at face of all other
supports..... $W_u \ell_n / 2$
