



**ALMUSTAQBAL UNIVERSITY COLLEGE**  
**DEPARTMENT OF BUILDING & CONSTRUCTION**  
**ENGINEERING TECHNOLOGY**  
**ANALYSIS AND DESIGN OF REINFORCED CONCRETE STRUCTURES II**

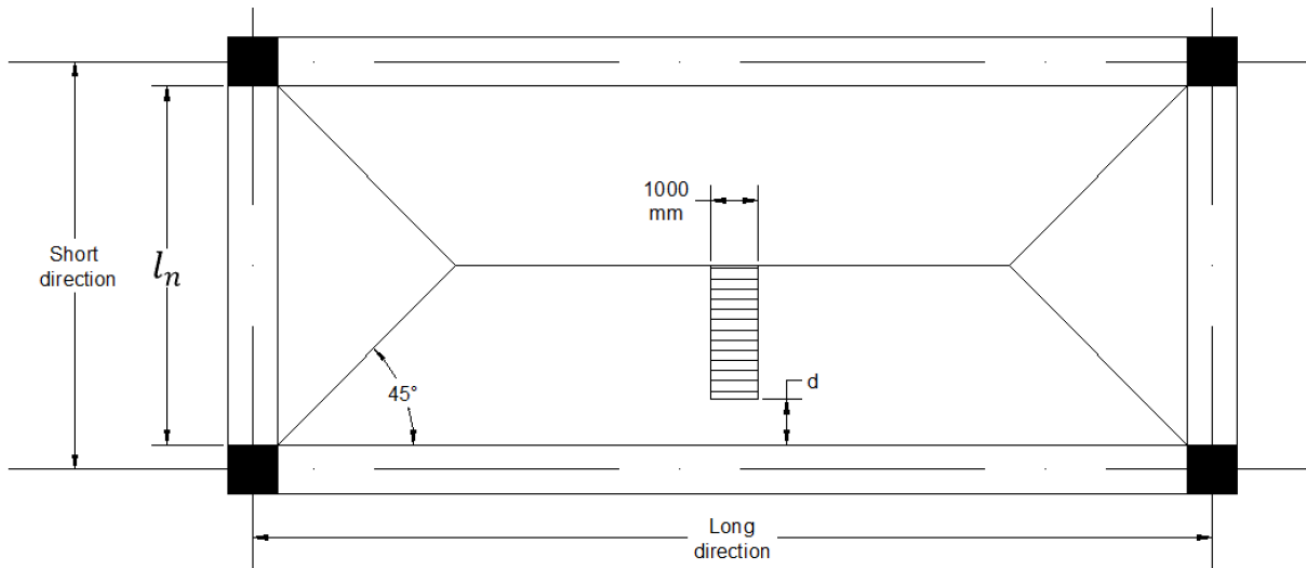
**SHEAR IN SLABS**

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## SHEAR IN SLABS

### 1. ONE WAY SHEAR:

#### A. SHEAR IN SLABS WITH BEAMS:



**Shear Force ( $V_u$ )** is the shear force caused by the load affecting the slab at a distance equal to the effective depth ( $d$ ) from the face of the beam.

$$V_u = W_u \left( \frac{l_n}{2} - d \right)$$

**Where:**

$V_u$ : ultimate shear force caused by the load.

$W_u$ : ultimate load.

$l_n$ : length of clear span from face to face of the columns.

$d$ : effective depth.

The ultimate shear force  $V_u$  should be equal or less than the factored concrete shear strength  $V_c$ .

$$V_u \leq \phi V_c$$

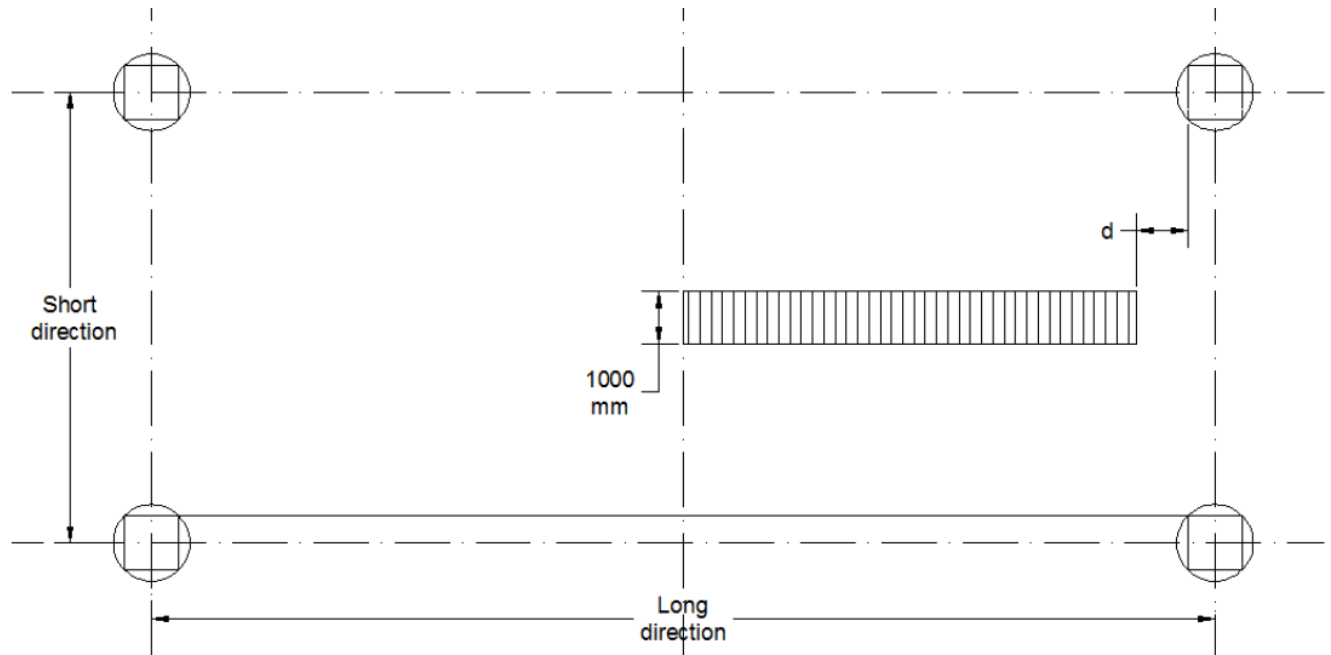
**Where:**

$$V_c = \frac{\sqrt{f_c'}}{6} b d$$

$$\phi = 0.75$$

$$b = 1000 \text{ mm}$$

## B. SHEAR IN SLABS WITHOUT BEAMS:



$$d_s = t - 20 - 1.5 D_{bar}$$

$$V_u \leq \phi V_c$$

$$V_c = \frac{\sqrt{f_c'}}{6} b d$$

$$B=1000mm, \phi = 0.75$$

## STEPS TO FOLLOW WHEN DETERMINING ONE WAY SHEAR SLAB

1. To find the shear force  $V_u$  we must determine  $l_n$  and  $d$ . determine  $l_n$  by taking the short span or long span which is required then subtract the dimension of the column.
2. The effective depth  $d$ , either  $d_{short}$  or  $d_{long}$  according to the required direction.

$$d_{short} = t - c - 0.5 D_{bar}$$

$$d_{long} = t - c - 1.5 D_{bar}$$

3. We determine the shear force  $V_u$  using the formula  $V_u = W_u \left( \frac{l_n}{2} - d \right)$ .
4. We determine the factored concrete shear  $\phi V_c$

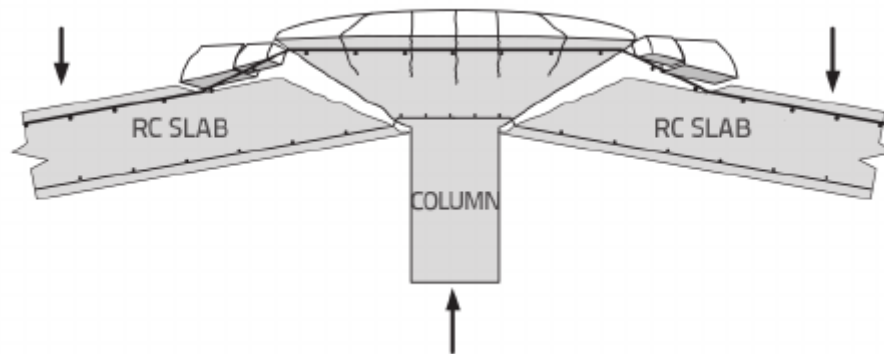
$$V_c = \frac{\sqrt{f_c'}}{6} b d, \phi = 0.75 \text{ \& } b=1000mm$$

5. Compare  $V_u$  to  $\phi V_c$ .

$$V_u \leq \phi V_c$$

## 2. TWO WAY SHEAR (PUNCHING SHEAR)

- **Punching shear** is a type of failure of reinforced concrete slabs subjected to high localized forces.
- In flat slab structures this occurs at column support points.
- This type of failure is catastrophic because no visible signs are shown prior to failure.
- Punching shear failure disasters have occurred several times in this past decade.



## METHODS USED TO PREVENT PUNCHING SHEAR FROM OCCURRING

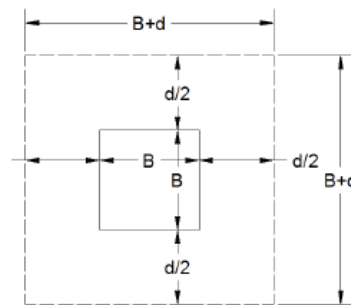
1. Increasing the thickness of slab or increasing the column dimensions (uneconomical).
2. Using drop panels.
3. Using column capital
4. Using shear reinforcement.

**Check the examples below.**



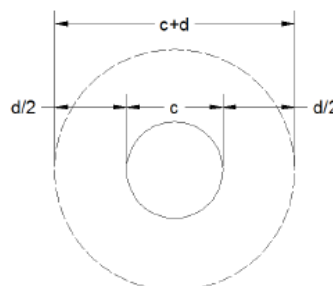
## HOW TO CALCULATE PUNCHING SHEAR

- Critical section for the punching shear is taken perpendicular to the plane of the slab at a distance  $\frac{d}{2}$  from the support.
- Area defined by the critical shear perimeter is  $b_o$ .
- For a rectangular column:



$$b_o = (B + d) \times 4$$

- For a circular column:



$$b_o = \pi (c + d)$$

- Determine the shear force  $V_u$ .

$$V_u = W_u (l_1 \times l_2 - A_{ps})$$

- Determine the shear stress  $v_u$

$$v_u = \frac{V_u}{b_o d}$$

- Determine concrete shear  $v_c$

	$0.33 \lambda \sqrt{f'_c}$
$v_c$ is the <b>least</b> of	$0.17 (1 + \frac{2}{\beta}) \lambda \sqrt{f'_c}$
	$0.083 (2 + \frac{\alpha_s d}{b_o}) \lambda \sqrt{f'_c}$

Where:

$\lambda = 1$  for normal concrete.

$\beta$  = is the ration between the long side to the short side of the column.

$\alpha_s$  = position of column, interior column  $\alpha_s = 40$ , edge column  $\alpha_s = 30$ , corner column  $\alpha_s = 20$ .

- If  $\phi v_c \geq v_u$ , then **no shear reinforcement** is required.
- If otherwise  $\phi v_c < v_u$ , then **shear reinforcement** is required.
- Check if  $v_u \leq \phi \times 0.5 \times \sqrt{f'_c}$
- We must determine  $v_s$

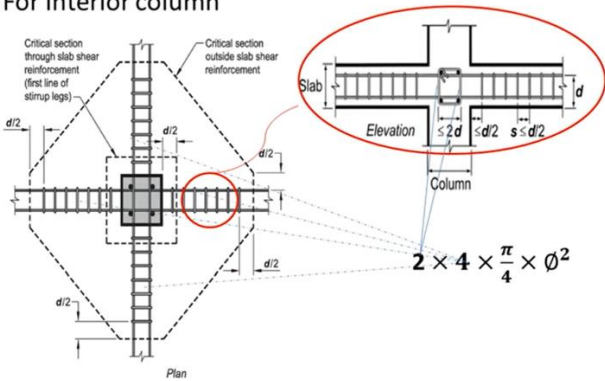
$$v_s = \frac{v_u}{\phi} - v_c, \text{ where } v_c = 0.17 \lambda \sqrt{f'_c}$$

- Determine the shear reinforcement area ( $A_v$ ) or spacing between stirrups ( $s$ ) from the following formula:

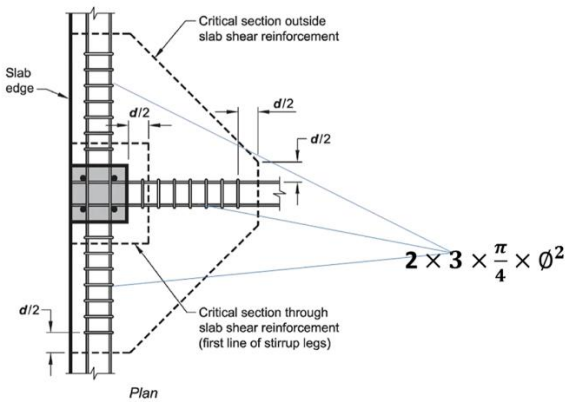
$$v_s = \frac{A_v f_y}{b_o s}$$

- If  $A_v$  is required, we use  $s = \frac{d}{2}$ .
- If  $s$  is required, then  $A_v$  is determined according to the position of the column.

- Calculating of  $A_v$
- For interior column



**Edge Column**



**Corner Column**

