## **Normal Stresses**

Stress is defined as the strength of a material per unit area of unit strength. It is the force on a member divided by area, which carries the force, formerly express in psi, now in  $N/mm^2$  or MPa.



where  $\mathbf{P}$  is the applied normal load in Newton and  $\mathbf{A}$  is the area in mm<sup>2</sup>. The maximum stress in tension or compression occurs over a section normal to the load.

Normal stress is either tensile stress or compressive stress. Members subject to pure tension (or tensile force) is under tensile stress, while compression members (members subject to compressive force) are under compressive stress.

Compressive force will tend to shorten the member. Tension force on the other hand will tend to lengthen the member.

## **Normal Strain**

Let us suppose that the bar of Fig. 1-1 has tensile forces gradually applied to the ends. The elongation per unit length, which is termed normal strain and denoted by  $\Delta$ , may be found by dividing the total elongation  $\Delta$  by the length L, i.e.,

$$\varepsilon = \frac{\Delta}{L}$$

The strain is usually expressed in units of meters per meter and consequently is **dimensionless**.

## **Stress-Strain Curve**

As the axial load in Fig. 1 is gradually increased, the total elongation over the bar length is measured at each increment of load and this is continued until fracture of the specimen takes place. Knowing the original cross-sectional area of the test specimen, the normal stress, denoted by s, may be obtained for any value of the axial load by the use of the relation

$$\sigma = \frac{P}{A}$$

experimental data may be plotted with these quantities considered as ordinate and abscissa, respectively. This is the stress-strain curve or diagram of the material for this type of loading. Stress-strain diagrams assume widely differing forms for various materials.



Ex; In Fig, determine an expression for the total elongation of an initially straight bar of length  $\mathbf{L}$ , cross-sectional area  $\mathbf{A}$ , and modulus of elasticity  $\mathbf{E}$  if a tensile load  $\mathbf{P}$  acts on the ends of the bar.



Ex; a surveyor steel tape of 30m length has a cross-section of 0.5mm thickness and 15mm width. determine the elongation when the entire tape is stretched and held taut by a force of 1kN, the modulus of elasticity = 207 GPa.

$$E = \frac{\sigma}{E} = \frac{P/A}{AL/L} = \frac{P * L}{A * AL}$$

$$\therefore \Delta L = \frac{PL}{AE}$$

$$L = 30 \text{ m} = 30 * 10^{3} \text{ mm}$$

$$P = 1 \text{ KN} = 1000 \text{ N}$$

$$A = 0.5 * 15 = 7.5 \text{ mm}^{2}$$

$$E = 207 \text{ GPu} = 207 * 10^{3} \text{ MPu}$$

$$\therefore \Delta L = \frac{PL}{AE} = \frac{1000 \times 30 \times 10^{3}}{7.5 \times 207 \times 10^{3}} = \frac{19.32 \text{ mm}}{19.32 \text{ mm}}$$

Ex; determine the elongation in the system shown

Sol:  
D) For section (a-a)  

$$\Sigma Fx = 0$$
  
 $0 = 100 - P$   $\therefore P = 100 KN \ll$   
 $\therefore \Delta L = \frac{PL}{AE} = \frac{100 \times 10^3 \times 1.5 \times 10^3}{500 \times 8.2 \times 10^3}$   
 $= 36.58 \text{ mm}$  elongation



Ex; find the maximum allowable value of (P) for the column shown in figure. The maximum allowable stress in steel = 120MPa, in timber is 12 MPa and in concrete is 16 MPa, Es= 200 GPa, Et =8.2 GPa, Ec = 17 GPa  $_{2P|}$ 



2) section (b-b)  

$$\begin{aligned}
& \xi Fy = 0 \\
& 0 = -2P + P + Y \\
& \hline & \swarrow Y = P \\
& \hline & Compression
\end{aligned}$$

$$\begin{aligned}
& \mathcal{S}_{timper} = \frac{F_{orce}}{A} \\
& 12 = \frac{P}{2000} \implies P = 24000 N = 24 KN \\
\end{aligned}$$
3) section (c-c)  

$$\begin{aligned}
& \xi Fy = 0
\end{aligned}$$

2P

Ρ

y

2P

Ρ

4P

y

$$2 Fy = 0$$
  

$$0 = -2P + P - 4P + Y$$
  

$$\therefore Y = 5 \cdot P \quad Compression$$
  

$$Oconcrete = \frac{Force}{A}$$
  

$$16 = \frac{5P}{8000} \implies P = 25600 \quad N = 25.6 \quad KN$$
  

$$\therefore P = 24 \quad KN$$

$$\frac{T_{o} \ Find \ \Delta L_{1,-}}{\Delta L_{s} = \frac{F_{ovce} \ * \ L_{s}}{A_{s} \ * \ E_{s}} = \frac{2 \ * \ 24 \ * \ 10^{3} \ * \ 1.5 \ * \ 10^{3}}{5_{oo} \ * \ 200 \ * \ 10^{3}} = 0.72 \ mm$$

$$\Delta L_{z} = \frac{F_{ovce} \ * \ L_{z}}{A_{z} \ * \ E_{z}} = \frac{24 \ * \ 10^{3} \ * \ 1 \ * \ 10^{3}}{2_{000} \ * \ 8.2 \ * \ 10^{3}} = 1.463 \ mm$$

$$\Delta L_{c} = \frac{F_{ovce} \ * \ L_{c}}{A_{c} \ * \ E_{c}} = \frac{5 \ * \ 24 \ * \ 10^{3} \ * \ 1.5 \ * \ 10^{3}}{8_{ooo} \ * \ 17 \ * \ 10^{3}} = 1.32 \ mm$$

$$\therefore \ T_{o}tal \ \Delta L = 0.72 \ + \ 1.463 \ + \ 1.32 \ = 3.5 \ mm$$

Ex; A rigid block of mass (M) is supported by three symmetrically spaced rods as shown in fig. Each copper rod has an area of 900 mm<sup>2</sup>; E = 120 GPa; and the allowable stress is 70 MPa. The steel rod has an area of 1200 mm<sup>2</sup>; E = 200 GPa; and the allowable stress is 140 MPa. Determine the largest mass (M) which can be supported.

Sol:  
2 Perper + Psterl = (M) - --O  

$$\Delta L_{steel} = \Delta L_{copper}$$
  
 $\frac{P_{steel} + L_{steel}}{A_{steel}} = \frac{P_{c-PPer} + L_{copper}}{A_{copper} + L_{copper}}$   
 $\frac{P_{steel} + L_{steel}}{T_{200} + 200 \times 10^3} = \frac{P_{copper} \times 160}{900 \times 120 \times 10^3}$   
 $1 \neq 15^{c}$   $P_{steel} = 1,481$   $P_{copper}$   
 $Sub in O$   
 $2P_{copper} + 1,481$   $P_{copper} = (M) - -O$   
 $Z + (\frac{P_{steel}}{1,481}) + P_{steel} = (M) - O$   
 $O = \frac{P_{copper}}{A_{copper}}$   
 $\therefore P_{copper} = O = O = O P e r + A = 70 + 900 = 63000 N$   
 $G = \frac{P_{steel}}{A_{steel}}$   
 $\therefore P_{steel} = 15teel + A steel = 1200 \times 140 = 168 \times N$   
 $M_{2} = 2 \times (\frac{168}{1,481}) + 168 = 394,8 \times N$   
 $\therefore M = 219,303 \times N$ 

...

Propper

Ex; Determine the largest weight W that can be supported by two wires shown in Fig. The stress in either wire is not to exceed 30 MPa. The cross-sectional areas of wires AB and AC are 40 mm<sup>2</sup> and 50 mm<sup>2</sup>, respectively.

$$\frac{5 \cdot L}{\xi F_{X} = 0}$$

$$0 = -AB * Cos 3 \circ + A_{c} * Cos 5 \circ$$

$$0 = -AB * 0,864 + A_{c} * 0,642$$

$$AB * 0,864 = Ac * 0,642$$

$$(\cdot, AB = Ac * 0,741) \text{ or } Ac = AB * 1,349$$

$$\xi F_{Y} = 0$$

$$0 = -W + AB * 5in 3 \circ + Ac * 5in 5 \circ$$

$$0 = -W + 0,5 * AB + 0,766Ac$$

$$(\cdot, W = 1,136 Ac) = 0$$

$$F_{0} = AB$$

$$(\cdot, 3) = \frac{AB}{Y_{0}} \implies AB = 3 \circ * 40 = 12 \circ N = 1.2 KN$$

$$(\cdot, W = 1,533 + 1.2 = 1,839 EN$$

$$F_{0} = AC$$

$$(\cdot, W = 1,136 + 1.5 = 1.5 \circ N = 1.5 KN$$

$$(\cdot, W = 1,136 + 1.5 = 1.5 \circ N = 1.5 KN$$

$$(\cdot, W = 1,136 + 1.5 = 1.5 \circ N = 1.5 KN$$

$$(\cdot, W = 1,136 + 1.5 = 1.5 \circ N = 1.5 KN$$

:

Ex; The assembly in Fig. P-242 consists of a light rigid bar AB, pinned at O, that is attached to the steel and aluminum rods. In the position shown, bar AB is horizontal and there is a gap,  $\Delta = 5$  mm, between the lower end of the steel rod and its pin support at C. Compute the stress in the aluminum rod when the lower end of the steel rod is attached to its support.

